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Benjamin Edwards, John Douglas

▶ To cite this version:

Benjamin Edwards, John Douglas. Selecting ground-motion models developed for induced seismicity in geothermal areas. Geophysical Journal International, 2013, 195 (2), pp.1314-1322. 10.1093/gji/ggt310. hal-00849230

HAL Id: hal-00849230 https://brgm.hal.science/hal-00849230

Submitted on 30 Jul 2013

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- **1** Selecting ground-motion models developed for induced seismicity in geothermal areas
- 2 Benjamin Edwards¹ and John Douglas²
- ¹Swiss Seismological Service, ETH, Zürich, Switzerland
- 4 ² Seismic and Volcanic Risks Unit, Risks and Prevention Division, BRGM, Orléans, France
- 5 Accepted --/----. Received --/----; in original form 05/2013.
- 6 *Abbreviated title*: Selecting ground-motion models
- 7
- 8 *Corresponding author:*
- 9 Benjamin Edwards
- 10 Swiss Seismological Service
- 11 Eidgenössische Technische Hochschule (ETH) Zürich
- 12 Sonneggstrasse 5
- 13 8092 Zurich, Switzerland
- 14 Email: <u>edwards@sed.ethz.ch</u>
- 15 Tel: +41 44 632 89 63

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20 Summary

21 We present a case study of the ranking and weighting of ground-motion prediction equations (GMPE) 22 for seismic hazard assessment of Enhanced Geothermal Systems (EGSs). The study region is Cooper Basin (Australia), where a hot-fractured-rock project was established in 2002. We test the applicability 23 24 of 36 GMPEs based on stochastic simulations previously proposed for use at EGSs. Each GMPE has a 25 set of corresponding model parameters describing stress drop, regional and local (near-surface) attenuation. To select suitable GMPEs for Cooper Basin from the full set we applied two methods. In 26 27 the first, seismograms recorded on the local monitoring network were spectrally analysed to determine 28 characteristic stress and attenuation parameters. In a second approach, residual analysis using the log-29 likelihood (LLH) method was used to directly compare recorded and predicted short-period response 30 spectral accelerations. The resulting ranking was consistent with the models selected based on spectral 31 analysis, with the advantage that a transparent weighting approach was available using the LLH 32 method. Region-specific estimates of variability were computed, with significantly lower values 33 observed compared to previous studies of small earthquakes. This was consistent with the limited range of stress-drops and attenuation observed from the spectral analysis. 34

Keywords: geothermal power, induced seismicity, ground-motion prediction, seismic hazard, spectralanalysis

37 Introduction

Ground shaking from seismicity associated with stimulation and exploitation of a geothermal reservoir 38 39 for heat and power production can be a significant nuisance to the local population and can, in some 40 cases, lead to building damage. The Deep Heat Mining project (Basel, Switzerland) in 2006 triggered an M_L 3.4 (M_w3.2) mainshock and thousands of smaller shocks and led to insurance claims of more 41 42 than \$9 million (Giardini, 2009). Two earthquakes (M_L 2.4 and 2.7) occurred in the vicinity of the Landau (Germany) geothermal power plant in 2009, which caused macroseismic intensities up to V+, 43 while at another German geothermal project (Insheim) two felt tremors (M_L 2.2 and 2.4) occurred 44 during reservoir stimulation in 2010 (Groos et al., 2013). Most recently, in July 2013, a geothermal 45 project in St. Gallen, Switzerland triggered a widely felt M_L3.5 (M_w3.4) event, which was followed by 46 47 numerous smaller aftershocks. In Majer et al. (2012) seven steps are proposed to help assess and 48 mitigate the seismic risk posed by geothermal systems. Step 5 of the proposal is to "quantify the 49 hazard from natural and induced seismic events" through either probabilistic or deterministic 50 approaches. They suggest a two-stage approach to quantify the hazard: a baseline estimate initially 51 established through regional seismicity, with further refinement to a site-specific hazard assessment 52 through the analysis of induced seismicity recorded on the local monitoring network. This article addresses the second stage. 53

A necessary component of any study that seeks to assess the seismic hazard (and/or risk) associated 54 with geothermal projects is a ground-motion model that estimates measures of shaking [e.g. peak 55 56 ground acceleration, (PGA)] given an earthquake scenario (e.g. in terms of magnitude and source-to-57 site distance). Motivated by a lack of suitable models in the public literature, Douglas et al. (2013) 58 derived a set of stochastic and empirical ground-motion models for application in geothermal areas. These models were based on analyses of thousands of near-source seismograms of small earthquakes, 59 60 most of which were induced by geothermal activity or gas extraction, while some were natural shallow 61 events.

62 Because of the considerable epistemic uncertainty in the estimation of ground motions in geothermal 63 areas, Douglas et al. (2013) presented ground-motion prediction equations (GMPEs) for 36 stochastic 64 simulation models (Boore, 2003) that sought to capture this uncertainty. The ranges of the key 65 parameters of these stochastic models (stress parameter, $\Delta \sigma$; path attenuation, Q; near-surface 66 attenuation, κ) were defined based on the analysis of seismograms collected from numerous regions. 67 The analysis highlighted considerable variation in these parameters among regions and sites. When conducting a seismic hazard assessment for a given geothermal project it is not known a priori which 68 of the 36 models are most applicable. Consequently Douglas et al. (2013) recommended, in the 69 70 absence of other information by which to constrain the stochastic parameters for a given site, that a 71 logic tree is used for seismic hazard analysis with all 36 models as branches. Subsequently, as 72 seismograms are recorded at the geothermal project site, the applicability of some models should 73 become evident and hence their associated branches could be assigned higher weights, while others may be down-weighted or even dropped completely. There are two complementary ways in which the 74 75 branch weights can be updated: direct assessment of the stochastic parameters, and comparison of 76 ground-motion predictions and observations.

77 As a demonstration of the use of the stochastic simulation GMPEs and the proposed procedure for the 78 assessment of the logic-tree weights, Douglas et al. (2013) present a simple analysis for Campi Flegrei 79 (Italy), for which some seismograms from small, shallow (but natural) earthquakes were available. 80 However, the small number of seismograms available (only 55), their limited bandwidths and low quality of the metadata meant that the logic-tree weights could not be significantly updated. The aim 81 82 of this article is to use a much larger and higher-quality dataset from an Enhanced Geothermal System 83 (EGS), to better demonstrate the proposed selection procedure and to investigate how many records are required to significantly reduce the epistemic uncertainty in ground-motion prediction for EGSs. 84 85 The dataset comes from Cooper Basin (Australia), which was not considered by Douglas et al. (2013) 86 when developing their ground-motion models.

The next section summarizes the 36 stochastic models developed by Douglas et al. (2013). Douglas etal. (2013) did not recommend their empirical models for application because they were derived using

data from a narrow magnitude-distance range (roughly M_w 1 to 3 and R_{hvp}<10km) and a simple 89 90 functional form was adopted. Because the stochastic models were based on simulated ground motions 91 from a wider range of magnitudes and distances and a more complex functional form was fit to these 92 simulations, they are considered more robust than the empirical equations for $M_w 1$ to 5 and $R_{hvp} 1$ to 93 50km. As with empirical models, however, extrapolation of stochastic models outside their range of applicability is not recommended. Following the introduction of the models, the dataset from Cooper 94 95 Basin EGS is presented. The subsequent section derives estimates of the stochastic parameters from 96 these data and investigates the impact of the number of seismograms used to estimate the parameters. 97 Based on this analysis a set of weights for the 36 models is proposed. A second set of weights is 98 proposed in the following section based on comparisons between the observed response spectral 99 accelerations and those predicted by the 36 GMPEs.

100 Ground-motion models for induced seismicity

Ground motions from small earthquakes, particularly those recorded in the near-source region, often 101 102 exhibit large variability for a given magnitude and distance, the principal independent parameters for GMPEs. There are two explanations for this common observation: the first is related to the fact that 103 meta-data for small earthquakes are often poorer quality (e.g., routine automatic locations as opposed 104 105 to manually-reviewed locations). The second reason (exacerbated by the first) is that ground motions 106 from small events are more sensitive to changes in hypocentral depth, while site-attenuation (κ) tends 107 to filter out, to varying degrees, the dominant high-frequencies associated with smaller earthquakes 108 (Douglas and Jousset, 2011). Furthermore, it is often observed that the variability of the stress-drop (or conversely, slip velocity) is significantly higher for small events than for larger events (e.g., Cotton et 109 al., 2013). Whether an artefact of inversion procedures (e.g., not properly accounting for attenuation), 110 or reality, this nevertheless reflects the greatly differing proportion of high-frequency energy observed 111 112 in small earthquakes of similar size.

113 Analysing data from six independent regions (Basel, Soultz, Geysers, Hengill, Roswinkel and Vorendaal), Douglas et al. (2013) found that a significant reduction in the overall prediction 114 uncertainty was obtained by accounting for region-specific biases. As discussed, this can be 115 interpreted as either systematic bias in the meta-data, or alternatively, region-specific 'characteristic 116 117 seismicity' and recording conditions. Since magnitudes were recomputed homogeneously and hypocentral depths are generally well-constrained for sources directly below the recording network 118 119 (typical in geothermal installations), Douglas et al. (2013) suggested that differences in source, path 120 and site conditions were the likely cause of region-specific differences. They proposed a suite of 36 121 stochastic models with different source, path and site properties to cover the range observed in their 122 datasets.

In terms of seismic hazard assessment, these 36 models can be considered to cover the epistemic 123 124 uncertainty: in the case of a completely unknown site, we cannot distinguish between any of the models, and must weight them equally. In reality, of course, some information about the site of interest 125 126 will always be available: for instance, if the EGS is not located on outcropping hard-rock, then we can 127 already rule out the models with the lowest levels of site attenuation (κ). Each of the 36 models has its own associated aleatory variability, which corresponds to a region-specific model. An initial 'naïve' 128 129 application of these GMPEs would be similar in terms of the resulting mean hazard to using the single 130 empirical GMPE developed by Douglas et al. (2013) using data from all regions (along with the 131 associated high aleatory variability). In terms of the hazard distribution, the obvious difference is that 132 the empirical model leads to only one curve, whilst the stochastic models generate 36 individual 133 hazard curves, representing the epistemic uncertainty. However, in the case of improved knowledge of the site's seismicity, or recording conditions, we can begin to reassess the weighting of the 36 models, 134 135 reducing the epistemic uncertainty. The advantage in this case is clear, since region-specific GMPEs 136 cannot typically be assessed due to limited recorded distance and magnitude ranges, the stochastic-137 model approach allows us to refine the logic tree in the case of improved knowledge, which will be quickly available after the installation of a monitoring network, or even beforehand when local data 138 139 already exist.

140 Ground-motion data used for model selection

141 The data used in this article comes from a sensitive seismic network set up to monitor the geothermal 142 exploitation of the reservoir at Cooper Basin (South Australia). A hot-fractured-rock project was launched at Cooper Basin in 2002 to exploit the Habanero granite reservoir at depths between 4 and 143 144 4.5km. Various boreholes and stimulation experiments have been conducted since and triggered 145 earthquakes have been located and characterized (Baisch et al., 2006). The data used here come from 146 2005 (data from 2003 are available but there is uncertainty over the calibration factors of the seismometers). Data from eight stations (Stang, 2011) installed by Q-Con [McLeod #1 (MCL), WA1-147 4 and MW1-3] are used (Figure 1), all of which are located below the surface (all at depths of less than 148 149 357m, except for McLeod #1 at 1.8km). High-quality earthquake catalogues were provided by Q-Con 150 for these data. The available records come from earthquakes with moment magnitudes between 1.7 151 and 3.1 (roughly following a standard Gutenberg-Richter distribution), hypocentral distances between 152 2.4 and 7.8km (roughly uniformly-distributed) and depths between 3.9km and 4.5km (roughly 153 normally-distributed with a peak around 4.2km). All records have been converted from velocity to acceleration through time-domain differentiation and application of calibration factors. Following the 154 155 approach detailed in Douglas et al. (2013) and Edwards and Douglas (2013), all earthquakes used here have had their moment magnitudes consistently recalculated. 156

157 Estimation of stochastic parameters

As noted, the estimation of suitable stochastic model parameters allows the reduction of epistemic 158 uncertainty related to ground-motion prediction in the study region for the magnitude and distance 159 range of the data. For earthquakes outside the range of observations (particularly larger magnitudes), 160 161 however, there still remains considerable epistemic uncertainty because it is not certain that the most appropriate stochastic parameters (or the best-fitting models) for the available data necessarily apply 162 for such scenarios. In a first step we look for existing parameters published in the literature. At Cooper 163 Basin, Baisch et al. (2009) found an average whole-path $Q_p=112\pm21$ and $\Delta\sigma=4.7$ bars (0.47MPa) from 164 the analysis of over 6000 events. Assuming $Q_p \approx Q_s$ and taking the measured $V_P = 3.7 \text{kms}^{-1}$ and 165 $V_P:V_S=1.9$ from Baisch et al. (2009) we obtain path attenuation (t*) of approximately 0.02s between 166 167 the event cloud and the surface. Due to the almost vertical propagation through the basin, this t* can 168 be completely assigned to the site-specific term, κ_0 . Regional values of Q have been computed for Australian earthquakes by Allen et al. (2006, 2007). For southeastern Australia Allen et al. (2007) 169 170 found:

 $log Q(f) = 3.66 - 1.44 \log f + 0.768 (\log f)^2 + 0.058 (\log f)^3$ 1 for frequencies (f) 0.78 to 19.9 Hz, leading to consistently high Q (1063 to 6671), whilst for southwestern Australia Allen et al. (2006) found:

$$Q(f) = 457f^{0.37}$$

2

for frequencies 1.07 to 25.0 Hz. The study of Allen et al. (2007) computed a corresponding 173 geometrical attenuation of $r_{hyp}^{-1.3}$ in the first 90km (where r_{hyp} is hypocentral distance) based on the 174 decay of long-period displacement spectra. The central-east location (Figure 1) of Cooper Basin may 175 be slightly better described by 'southeastern Australia'. However, in light of the known trade-off 176 177 between Q and geometrical decay, we may prefer to use the latter Q estimate for southwestern Australia, which corresponds to the r_{hyp}^{-1} geometrical spreading model adopted by Douglas et al. 178 (2013). This frequency dependent Q model (Equation 2) corresponds to 457 at 1Hz and 1503 at 25Hz, 179 180 which could foreseeably be accommodated through frequency-dependent weighting of the Douglas et 181 al. (2013) models developed with frequency independent Q. Stress drop terms derived by Allen et al. (2006) lie between 1 and 100 bars (0.1 and 10MPa) but show a trend that increases with magnitude, 182 albeit weakly. 183

184 To provide further estimates of the parameters, without the numerous assumptions that may be 185 required by using values from the literature, we here also present the average Q, $\Delta\sigma$ and κ_0 values as 186 determined from subsets of the data used for this study. We test the impact of using 10, 25 and 50% of 187 all data in order to simulate the effect of a developing database for a new network installation.

188 *Kappa estimation*

We estimated site κ_0 consistent with the models of Douglas et al. (2013) through least-squares 189 190 minimization of spectra computed over the duration of shaking. The window duration was based on 5-95% of the velocity-squared integral, with spectral models fit between 10 and 100Hz in the lin-log 191 domain. The models were based on the Brune (1970) source with single event-common corner-192 frequency and attenuation modelled using an exponential function (Anderson and Hough, 1984); refer 193 to Douglas et al. (2013) and Edwards et al. (2011) for an exhaustive description of the fitting 194 195 procedure. At least two instruments were required to have recorded each event to include it in the 196 processing. To estimate the impact of limited data on the choice of model weights, we simulated 197 different stages of data collection through random sampling of the events for which we had good 198 recordings at two or more instruments.

199 Since the hypocentral distance was very limited, choice of Q has a minimal impact on kappa: we chose 200 Q=1200 to be broadly consistent with the results of Allen et al. (2006), who found values of between 201 457 at 1Hz and 1503 at 25Hz. However, choosing the Q=600 model from Douglas et al. (2013) would 202 only have an impact of approximately $\Delta \kappa_0 = 0.001s$ (2-4%). We observe that the kappa values 203 determined for the Cooper Basin stations were dependent on the location of the borehole. Instrument 204 McLeod #1, located at the centre of the network (1.8km depth) has the lowest value (0.028s), consistent with the significant depth at which it is located. Stations MW01, MW02 and MW03 (the 205 206 inner ring) lie within approximately 2.5km of the central station (depths 109 to 357m) and show the 207 highest kappa values (0.041 to 0.047s). Stations WA01, WA02 and WA03 (outer ring) lie within 5km 208 of the central station and show moderate values of kappa (0.03 to 0.037s), despite being located 209 shallower than stations MW01, MW02 and MW03 (at depths of 96 to 110m). The similarity of the kappa values for each ring of the network is remarkable, and may be due to similar geology for these 210 211 stations or, since the earthquakes are all located near to the well-head, due to the similarity of the 212 propagation paths to each of the stations of a given ring.

We estimated the impact of reduced datasets by bootstrapping 100 times over random subsamples of the complete dataset; measuring the changes in absolute value and scatter. The impact of a reduced dataset (even down to 10% of the original events: corresponding to an average of 14 events) was minimal in terms of the average kappa, with changes of only a few per cent (Table 1). In the case of the standard deviation, the reduced datasets led to significant underestimation of the true uncertainty. This should not be an issue in our application, however, since we are interested in the median values; aleatory variability is independently assigned based on the work of Douglas et al. (2013).

220 Stress parameter estimation

Source corner frequencies of the spectra were re-estimated, fixing Q=1200 and κ_0 as in Table 1. An inversion was performed in the log-log domain, again minimizing the least-squares misfit of the spectral model. Given the moment magnitude determined by Edwards and Douglas (2013), we canthen estimate the stress-parameter as:

$$\Delta \sigma = M_0 \left(\frac{f_c}{0.4906\beta} \right)^3$$

where β =3500m/s and M₀ is estimated following the original formulation of the moment magnitude by Hanks and Kanomori (1979).

227 After selecting events with available M_w and f_c estimates a total of 95 earthquakes from Cooper Basin 228 were assigned stress parameters. The log-average was 19 bars with a standard deviation of 0.5 (ln 229 units; a factor of 1.65). Repeating the analysis with sub-selections of the events from Cooper Basin we 230 obtain standard deviations on both the mean and standard-deviation of the target value (Table 2). We see that even with only 25% of the events, the mean stress-parameter $\langle \ln[\Delta\sigma] \rangle$ (and the variability of 231 232 individual event stress-parameters $\sigma_{ln[\Delta\sigma]}$) is robust, with a variability (represented by the standard 233 deviation) of a factor of 1.12. Reducing the dataset to 10% (around 9-10 events) we begin to observe 234 larger (albeit not significant) deviations from the mean.

Based on the spectral analysis we can choose to assign weights to the models based on expert 235 236 judgement. We select models from Douglas et al (2013) with Q=600 and Q=1800 (covering the range 237 observed in the literature for this region), and based on the limited range of measured surface 238 attenuation values, κ =0.04s (Table 1). In order to cover the 19 bar average seen in the spectral 239 analyses, we then make a further selection of models with 10 and 100 bar stress-parameter. This leaves 240 four candidate models (Table 3). No preference in terms of weighting is given to GMPEs based on the 241 two different Q models due to the uncertainty of this parameter (Table 3). The final weighting of the 242 four selected models is then given by 0.365 for the two 10 bar and 0.135 for the two 100 bar models. These weights were chosen such that the log-average equivalent stress-parameter of the weighted 243 244 model was 19 bars: i.e., $2 \times 0.365 \times \log(10bar) + 2 \times 0.135 \times \log(100bar) \approx \log(19bar)$. The resulting 245 weighted stochastic GMPE is shown in Figure 2 for PGA along with predictions from the purely 246 empirical model of Douglas et al. (2013) and recorded data. The weighted stochastic model shows 247 better fit to the recorded data from small events, while for the few events with M \approx 3, both models 248 predict similar motions.

249 Residual analyses

In this section we analyse residuals computed from the 36 ground-motion models and data from Cooper Basin. 2089 pairs of horizontal time-histories were available for this analysis from the eight local stations and 427 earthquakes. From these time-histories the geometric-means of the pseudospectral accelerations (PSAs) for 5% damping from each pair are computed for 0.01s (100Hz, assumed equal to PGA) and 0.05s (20Hz) natural periods. The limited bandwidth of the seismometers installed

at Cooper Basin does not allow accurate PSAs to be computed for frequencies lower than about 15Hz. 255 Consequently the engineering use of these data is limited to examining the response to shaking of stiff 256 257 structures (e.g., low-rise masonry buildings) and non-structural elements. The observed and predicted 258 response spectra shown by Douglas et al. (2013) show that PSA(0.05s) is likely to be close to the peak 259 PSA for the magnitude range covered by the Cooper Basin data. In this section, these two sets of PSAs are statistically compared to the 36 ground-motion models proposed by Douglas et al. (2013). These 260 261 models consist of: the equations for the median PSAs derived using the stochastic method and the 262 aleatory-variability models for the single-station within-event standard deviation (φ_{SS}) (e.g., Al Atik et 263 al., 2010) and the zone-specific between-event standard deviation (τ_{zs}) equal to the average of the 264 models for this variability for the Soultz and Basel EGSs. The impact of changing the model for the 265 aleatory variability is investigated below.

266 Scherbaum et al. (2009) and Kale and Akkar (2013) propose methods to judge the applicability of 267 GMPEs to a given set of ground-motion data. These methods consist of the calculation of variables: 268 log-likelihood (LLH, Scherbaum et al., 2009) and Euclidian Distance Ranking (EDR, Kale and Akkar, 269 2013), which are both based on the differences between the natural logarithms of the observed and predicted PSAs, although the influence of the aleatory variability is different in the two cases. Because 270 271 we are assuming the same aleatory variability for all tested models we could simply use the 272 differences in mean residuals to rank the models but the use of the LLH values allows us to weight the 273 different models in a mathematically rigorous way (see below). The lower the value of LLH and EDR, 274 the closer the match between the observations and predictions. The large number of records available from Cooper Basin enables us to investigate the impact of the number of records available on the 275 276 results of the GMPE testing. Generally geothermal projects will have fewer records available with 277 which to judge the applicability of the available GMPEs (especially before stimulation or early on in 278 the stimulation phase) and hence it is useful to study whether the GMPE testing is sensitive to the 279 number of records used.

Based on comparisons between the LLHs and EDRs (and the implied ranking of models) computed 280 281 for PSA(0.01s) and PSA(0.05s) it was found that LLH and EDR are strongly linearly correlated for both periods, as are these values for the two structural periods. Therefore, for brevity, in the rest of this 282 283 section only LLHs (and GMPE ranking and logic-tree weights derived from these values) for 284 PSA(0.01s) are presented. LLH is preferred to EDR as a measure of the applicability of GMPEs because of the direct link between a set of LLHs and logic-tree weights in the case of well-distributed 285 data and similar model extrapolation behaviour (as is the case for physically-based stochastic models). 286 287 About a third of the seismograms from Cooper Basin required high-cut filters that removed ground 288 motions with periods below 0.05s (frequencies above 20Hz), which could be affecting PSA(0.01s). 289 However, repeating the analyses described below with and without these seismograms showed that the 290 influence of these band-limited records on the results is minimal. Therefore, all 2089 geometric-mean observations of PSA(0.01s) were used, from which normalized residuals were computed. For the total variability, we use a value of 0.96 (ln units), the value computed from the estimates of φ and τ given by Douglas et al. (2013).

294 To test the stability of the LLH values with respect to the number of records being used a bootstrap 295 procedure was followed whereby 100 random sets of 1044 (half the total), 522 (quarter of the total) and 261 (eighth of the total) samples are selected from the 2089 available and the analysis repeated. 296 297 From these results the mean and standard deviation (from the 100 results) of the LLH of each ground-298 motion model were computed (Table 4). As expected, the standard deviation increases as the number 299 of available records decreases. Surprisingly, however, the standard deviations remain low and 300 consequently the LLHs are stable, even when only an eighth of the records are used. This suggests that 301 even a few hundred seismograms would enable robust logic-tree weights to be computed for hazard 302 assessments of EGS projects if it were assumed that the highest-weighted models apply for 303 magnitudes and distances outside the range covered by observations.

It is interesting to note that the best-performing models are for values of $\Delta \sigma$, Q and κ_0 similar to those previously reported for the Cooper Basin area or calculated above. This suggests that logic-tree weights can be preliminarily assessed based on values of these key parameters taken from the literature (if they are mutually consistent) or from seismological analyses of data, without statistically comparing the observations and predictions.

309 The definition of LLH allows a direct computation of logic-tree weights (Scherbaum et al., 2009). 310 Such an approach is not necessarily appropriate in terms of a probabilistic seismic hazard assessment, 311 however, because the weights do not represent the probability of a given model being correct 312 (Delavaud et al., (2012). Instead, LLH-based weights represent a given model's ability to fit the 313 observed data, favouring 'better models'. Typically, the goal of a complete logic-tree based hazard 314 analysis is to capture not only the centre and body, but also the range of possibilities. In this sense the LLH weights, by design, will not cover the entire range (i.e., extreme scenarios not yet recorded). To 315 bypass this limitation, Delavaud et al. (2012), suggest weighting based on expert judgement, with help 316 317 from LLH information. However, a fully transparent approach for logic-tree weighting still does not 318 exist. Considering this limitation, we adopt the LLH-based logic-treeweights for this analysis. 319 Nevertheless, for the purpose of hazard assessment in geothermal zones, we would recommend further 320 expert elicitation to ensure that the complete range of possible models are appropriately considered in the logic tree. 321

Applying the equation of Scherbaum et al. (2009) to the values of LLH listed in Table 4 for the complete dataset gives the weights summarized in Figure 3, from which it can be seen that roughly half of the models contribute about 75% of the total weight. Modifying the standard deviation associated with each model from 0.96 [the sigma proposed by Douglas et al. (2013), for use with the

stochastic models] to 0.64 (the sigma obtained by regression analyses on the Cooper Basin data, see 326 327 below) does not alter the model ranking but it slightly increases the distinction between models. Therefore, the model weights are concentrated in the best ranked models (75% of the weight is 328 contributed by roughly a third of the models). In terms of computational efficiency the use of 36 329 models in a logic tree may be problematic. In this case it may be useful to trim the number of models 330 from the total 36 before weighting. As discussed previously, LLH weighting tends to favour models 331 332 which better predict the data. By using the LLH weighting of the full set of models it should, 333 therefore, be possible to find and remove models that do not add any information about possible 334 epistemic uncertainty. For instance, if the highest and second-highest weighted models predict very 335 similar ground-motions, then the second model can be removed without affecting the hazard results. 336 Such analysis is, however, beyond the scope of this article.

337 Aleatory Variability

To assess the aleatory variability of the Cooper Basin ground-motion data, random-effects regressionwas performed using the functional form of Model 1 of Douglas et al. (2013):

340
$$\ln \text{PSA}(0.01s) = a + bM_W + c \ln \sqrt{r_{hyp}^2 + h^2} + dr_{hyp}$$
 4

where PSA(0.01s) is in m/s², M_W is moment magnitude and a, b, c, d and h are regression coefficients. 341 The limited distance range of the Cooper Basin data does not allow robust estimates of h (describing 342 343 near-source saturation) and d (describing anelastic attenuation) to be found and, therefore, these coefficients are constrained to zero. The coefficients obtained from using the entire 2089 records are: 344 a=-6.899, b=2.569 and c=-2.589 with a between-event standard deviation (τ) of 0.099 and a within-345 even standard deviation (φ) of 0.627, leading to an overall standard deviation (σ) of 0.635. Comparing 346 347 these coefficients to those obtained by Douglas et al. (2013) from regression on data from six areas indicates slightly higher magnitude dependence (coefficient b) and faster attenuation (coefficient c) for 348 349 Cooper Basin data. The most noticeable difference, however, is the much smaller values of φ and, in 350 particular, τ , from regression on the Cooper Basin data compared to those obtained by Douglas et al. 351 (2013). The much lower value of τ can be partly explained by the use of data from a single zone (this 352 type of τ is called τ_{ZS} by Douglas et al., 2013) but it appears that ground motions at Cooper Basin are much less variable than those at other EGS sites; Douglas et al. (2013) found for Basel τ_{ZS} =0.637 and 353 354 for Soultz τ_{ZS} =0.902.

The value of τ for Cooper Basin is even lower than those associated with GMPEs derived from moderate and large earthquakes [see Figure 10c of Douglas et al. (2013), where τ for none of the considered GMPEs is lower than 0.2]. This very small τ can be related to the small variability in the stress drops of Cooper Basin: found to be 0.5 ln-units, corresponding to a factor of 1.65. This is

significantly lower than most other studies: for instance, Allmann and Shearer (2009) find a value of 359 360 1.46 ln-units for global intra-plate events; Edwards and Fäh (2013) find 1.83 ln-units and 1.43 ln-units 361 for the Swiss foreland and Alpine regions respectively; Oth et al. (2010) find 1.38 ln-units for 362 Japanese earthquakes and Rietbrock et al. (2013) find 1.38 ln-units for UK events. Cotton et al. (2013) showed that spectral analysis methods applied to small earthquakes often led to significantly larger 363 stress variability than seen in larger events. They presented the required variability in stress parameter 364 365 corresponding to the aleatory variability of several GMPEs and concluded that the variability should 366 lie between 0.26 and 0.59 ln-units for large events. The difference may relate to the strong regional 367 variability in source, path and site effects for small earthquakes, either real or due to parameter trade-368 off. Treated independently, the data from Cooper Basin (a limited source zone) and the consistency of 369 wave propagation may mean that the observed variability usually inherent with such small events is 370 not apparent. One issue to consider, in this case, is whether the observed variability truly reflects the 371 possible future variability: e.g., do we account for a near-surface event outside of the seismic cloud as 372 was the case for the largest event related to the Berlín (El Salvador) geothermal project (Bommer et 373 al., 2006), or of significantly different stress-drop?

374 Single-station φ was calculated for the eight Cooper Basin stations as $\varphi_{SS}=0.493$, which is again smaller than that obtained by Douglas et al. (2013), φ_{ss} =0.576. The relatively low value is consistent 375 376 with the limited range of station kappa values (with three distinct groups corresponding to the middle, 377 inner and outer stations). The total sigma (combining the between- and within-event variability) is, therefore, much lower than that for the empirical models of Douglas et al. (2013) and it is more in line 378 379 with those associated with GMPEs for moderate and large earthquakes. This demonstrates that at least 380 some of the large variability in the empirical models of Douglas et al. (2013) is due to mixing data 381 from various sites when deriving these models.

382 *Reduction of epistemic uncertainty*

383 As highlighted by Douglas et al. (2013) the disadvantages of applying their empirical GMPE are twofold. Firstly, the limited magnitude-distance range means that application to rarer, but potentially 384 damaging events, is tenuous and, secondly, the aleatory variability assigned to their equation was 385 386 strongly contaminated by epistemic uncertainty from combining several regional datasets (e.g., due to 387 differences in seismicity and attenuation). Effectively the empirical model can be thought of as a mixture model: comprising several different sets of source and propagation behaviour, but without 388 389 consideration of the increased sigma relative to a predictive relation. Nevertheless, it is not trivial to isolate such effects given limited recordings. Douglas et al. (2013) suggest that to reduce the 390 uncertainty, stochastic simulation models can be used. Of course, such models are not without 391 uncertainty outside their 'calibrated' model-space: the magnitude and distance range over which the 392 393 simulation model can be tested against recorded seismograms. However, unlike empirical models, due

to their physical basis, alternative models can be easily developed to cover the epistemic uncertainty outside the magnitude range available in instrumental databases. For this purpose, Douglas et al. (2013) provided 36 GMPEs to cover a range of simulation parameters: with various κ , Q and $\Delta \sigma$. A further benefit of testing and weighting simulation models, as performed here for Cooper Basin, is that it can help to limit the influence of epistemic uncertainty contamination related to mixing different sites.

400 The analysis undertaken here showed that the stochastic models can be selected based on spectral 401 analysis, or on LLH testing. Four stochastic models performed well in LLH testing, whist also having 402 stochastic model parameters consistent with the results of spectral analysis: Models 31, 23, 19 and 35. 403 These models, along with the empirical model of Douglas et al. (2013) and the best fitting empirical 404 model just using the Cooper Basin data are shown in Figure 4a. We see that the preferred stochastic 405 models are similar to the empirical model based on Cooper Basin data (Equation 4), while the best-406 fitting model for the dataset shows a slightly faster decay. By producing weights following Scherbaum 407 et al. (2009) the resulting weighted median stochastic model (from all 36 weighted component 408 models) is shown in Figure 4b. The weighted median model leads to lower PGA at M<3 (up to a 409 factor of 1.5) than the empirical model of Douglas et al. (2013), while the aleatory variability is 410 reduced by over 25%.

411 *Conclusions*

In this study we have taken an existing EGS site as a case study of the proposals by Majer et al. (2012) 412 413 and Douglas et al. (2013) to characterize the expected ground motions. To simulate the realistic 414 temporal development of a seismic database and corresponding earthquake catalogue, we randomly 415 resampled the full databases to one half, one quarter and one eighth of the full dataset. Following the 416 approach of Douglas et al. (2013), we then developed weighted median models to describe the sub-417 datasets. It was found that both spectral analysis for the stochastic model parameters and residual 418 analysis provided complementary results, with the highest weighted models from Douglas et al. (2013) 419 consistent with both existing literature and values determined here. Using the LLH method we were 420 able to automatically assign weights using a consistent and transparent approach. The resulting models 421 were shown to significantly reduce the epistemic uncertainty related to ground-motion prediction for 422 EGS projects.

423

424 Acknowledgments

This study was partially funded by GEISER (Geothermal Engineering Integrating Mitigation of
Induced Seismicity in Reservoirs) project funded under contract 241321 of the EC-Research Seventh
Framework Programme (FP7). We thank Geodynamics Ltd. (Australia) and QCon (Germany) for the

- 428 data from Cooper Basin. We thank Julian Bommer and an anonymous reviewer for their detailed429 comments on an earlier version of this article.
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495 *Tables*

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100% 50% 25% 10% Depth Δ{κ₀} **Δ{±σ}** ∆{#} Δ{κ₀} **Δ{±σ}** ∆{#} Δ{κ₀} $\Delta{\pm\sigma}$ ∆{#} Station [m] к [s] ±σ [s] # $\pm \sigma_{\rm e}$ [s] [%] [%] [%] [%] [%] [%] [%] [%] [%] MCL 1783 0.0277 0.0133 113 0.0013 0.31 -2.23 0.22 -76 -13.89 -91 -49 1.16 -1.09 97.5 0.0335 0.0159 WA01 48 0.0023 -0.27 -1.47 -51 2.51 -3.36 -76 -4.20 -22.33 -90 96 WA02 0.0371 0.0136 49 0.0019 0.81 -3.17 -51 0.28 -10.13 -77 0.30 -30.00 -91 0.0307 0.0158 WA03 110 81 0.0018 0.28 -0.58 -49 1.28 -2.46 -75 1.04 -9.48 -90 97.5 WA04 0.0380 0.0149 109 0.0014 -0.05 -0.10 0.26 -1.10 -76 3.34 -7.47 -90 -50 MW01 357 0.0470 0.0128 161 0.0010 0.16 -1.80 -2.81 -7.29 -50 0.47 -76 0.14 -90 109 MW02 0.0405 0.0217 223 0.0015 0.05 -0.80 -50 -0.65 -1.09 -76 -1.55 -3.41 -90 239 0.0469 0.0134 194 0.0010 MW03 0.03 -1.09 -49 0.60 -0.90 -76 0.45 -4.16 -90

497 Table 1 : kappa (and the standard deviation $\pm \sigma$; and standard error $\pm \sigma_e$) determined for the sites of Cooper Basin, along with the impact of reduced datasets.

 $\label{eq:statistical analysis of stress parameter. All values in natural log-scale. < ln[\Delta\sigma] > is the ln-average stress-parameter$

499 500 501 and $\sigma_{ln[\Delta\sigma]}$ is the standard deviation of the individual event stress-parameters; $\sigma_{<ln[\Delta\sigma]>}$ is the standard deviation of the mean and $\sigma_{\sigma ln[\Delta\sigma]}$ is the standard deviation of the standard deviation over 1000 randomizations.

Percent of Data	<ln[∆σ]> [bars]</ln[∆σ]>	<Δσ> [bars]	σ <ln[δσ]></ln[δσ]>	$\sigma_{\ln[\Delta\sigma]}$	$\sigma_{\sigma \ln[\Delta\sigma]}$
100	2.93	18.67	0.000	0.502	0.000
75	2.91	18.40	0.024	0.497	0.015
50	2.92	18.56	0.041	0.528	0.055
25	2.93	18.78	0.111	0.502	0.110
10	2.78	16.20	0.163	0.390	0.076

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504 Table 3: Weighting scheme for the selected models based on Q and $\Delta\sigma.$

Model	Model Description	Q Weight	Δσ Weight	Total Weight
19	Δσ=10bar; Q=600; κ =0.04s	0.25	0.365	0.365
23	Δσ=10bar; Q=1800; κ =0.04s	0.25	0.365	0.365
31	Δσ=100bar; Q=600; κ =0.04s	0.25	0.135	0.135
35	Δσ=100bar; Q=1800; κ =0.04s	0.25	0.135	0.135

Table 4: Mean LLHs and their standard deviations (computed using a bootstrapping procedure) for the 36 ground-motion

models and various proportions of the Cooper Basin dataset. Models indicated in bold are the twelve best performing

506 507 508 models whilst those in italics are the twelve worst performing.

Δσ	Q	κ _o	All data	Half	Quarter	Eighth	No.
1	200	0.005	2.1007	2.0969±0.0228	2.0978±0.0407	2.0992±0.0622	1
1	200	0.02	1.7971	1.7965±0.0129	1.7982±0.0233	1.7988±0.0335	2
1	200	0.04	2.6115	2.6129±0.0255	2.6157±0.0464	2.6159±0.0705	3
1	200	0.06	3.7752	3.7780±0.0384	3.7816±0.0694	3.7818±0.1068	4
1	600	0.005	3.7820	3.7755±0.0438	3.7750±0.0787	3.7788±0.1238	5
1	600	0.02	1.7664	1.7648±0.0147	1.7663±0.0257	1.7672±0.0381	6
1	600	0.04	2.2444	2.2453±0.0205	2.2482±0.0372	2.2479±0.0574	7
1	600	0.06	3.2774	3.2798±0.0337	3.2836±0.0609	3.2831±0.0955	8
1	1800	0.005	5.2068	5.1987±0.0584	5.1963±0.1035	5.2031±0.1685	9
1	1800	0.02	1.8220	1.8200±0.0172	1.8213±0.0299	1.8224±0.0453	10
1	1800	0.04	2.1426	2.1432±0.0190	2.1461±0.0344	2.1457±0.0533	11
1	1800	0.06	3.1200	3.1222±0.0321	3.1261±0.0580	3.1253±0.0916	12
10	200	0.005	4.5300	4.5227±0.0477	4.5232±0.0891	4.5248±0.1319	13
10	200	0.02	2.0125	2.0091±0.0223	2.0091±0.0417	2.0113±0.0631	14
10	200	0.04	1.7867	1.7857±0.0125	1.7865±0.0226	1.7889±0.0289	15
10	200	0.06	2.4931	2.4938±0.0242	2.4953±0.0427	2.4981±0.0603	16
10	600	0.005	8.9585	8.9475±0.0758	8.9457±0.1397	8.9512±0.2162	17
10	600	0.02	2.7308	2.7261±0.0348	2.7252±0.0636	2.7281±0.0997	18
10	600	0.04	1.7389	1.7372±0.0136	1.7378±0.0254	1.7398±0.0357	19
10	600	0.06	2.1503	2.1504±0.0189	2.1520±0.0331	2.1541±0.0462	20
10	1800	0.005	11.9403	11.9273±0.0952	11.9222±0.1716	11.9328±0.2782	21
10	1800	0.02	3.1481	3.1427±0.0406	3.1414±0.0734	3.1450±0.1168	22
10	1800	0.04	1.7710	1.7690±0.0156	1.7695±0.0290	1.7715±0.0424	23
10	1800	0.06	2.0488	2.0487±0.0171	2.0503±0.0300	2.0522±0.0416	24
100	200	0.005	8.3257	8.3160±0.0695	8.3176±0.1313	8.3168±0.1878	25
100	200	0.02	3.0079	3.0031±0.0367	3.0021±0.0695	3.0041±0.1045	26
100	200	0.04	1.7318	1.7298±0.0143	1.7292±0.0283	1.7324±0.0398	27
100	200	0.06	2.0443	2.0440±0.0175	2.0442±0.0306	2.0482±0.0384	28
100	600	0.005	16.2242	16.2098±0.1033	16.2079±0.1929	16.2123±0.2911	29
100	600	0.02	4.5644	4.5579±0.0529	4.5557±0.0985	4.5585±0.1523	30
100	600	0.04	1.9695	1.9666±0.0222	1.9656±0.0430	1.9685±0.0650	31
100	600	0.06	1.8382	1.8374±0.0136	1.8374±0.0246	1.8408±0.0298	32
100	1800	0.005	21.2923	21.2752±0.1278	21.2684±0.2330	21.2802±0.3724	33
100	1800	0.02	5.4541	5.4468±0.0607	5.4438±0.1120	5.4475±0.1760	34
100	1800	0.04	2.1350	2.1317±0.0260	2.1305±0.0499	2.1333±0.0766	35
100	1800	0.06	1.7930	1.7920±0.0130	1.7919±0.0242	1.7952±0.0299	36

510 *List of Figure Captions*

511 Figure 1: Map of microseismic monitoring network (McLeod #1: MCL, MW01-3 and WA01-3).

- 512 Catalogued events from 2005 are indicated by grey symbols, those used in the spectral analysis for 513 determination of κ and $\Delta \sigma$ are shown by black symbols.
- Figure 2: Comparison of median PGA predictions from the weighted stochastic GMPE (with weights
 based on spectral analysis) and the empirical model of Douglas et al. (2013).
- Figure 3: Ranking of models against cumulative weights for two different sigmas. See Table 4Table 2 for the correspondence between model number and parameters of the stochastic model. Grey numbers indicate models consistent with the spectral analysis and literature (Q equal to 600 to 1800; $\Delta \sigma$ equal
- 519 to 10 to 100 bars; and $\kappa 0 = 0.04$ s).
- Figure 4: (a) PGA data for Cooper basin for events with 2.5<M<3.1 and the selected predictions (stochastic and empirical) from Douglas et al. (2013) for M2.8 along with the best fit of the data to equation 3. (b) The empirical model of Douglas et al. (2013) along with the weighted median stochastic model (from LLH testing) including their variabilities.
- 524
- 525 Figures
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528Figure 1: Map of microseismic monitoring network (McLeod #1: MCL, MW01-3 and WA01-3). Catalogued events from5292005 are indicated by grey symbols, those used in the spectral analysis for determination of κ and Δσ are shown by black530symbols.

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al. (2013) along with the weighted median stochastic model (from LLH testing) including their variabilities.